

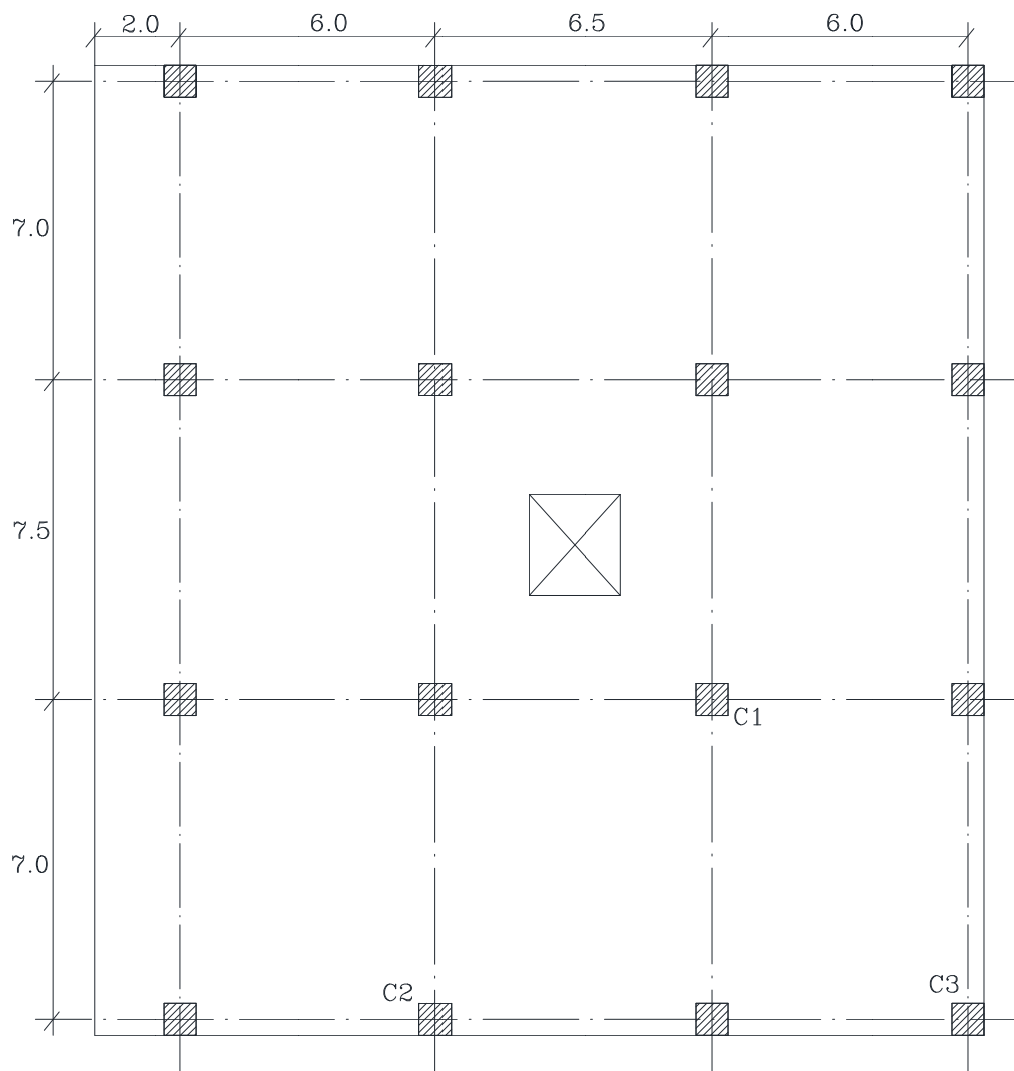


### **EXAMPLE**

The Given plan shows a general layout of flat slab.

#### **DATA**

F.C=1.5 kN/m<sup>2</sup>      L.L= 6 kN/m<sup>2</sup>  
No. of floors=4      Floor Height=4.50 m  
F<sub>cu</sub>=25 MPa      Steel used 360/520





## Concrete dimensions

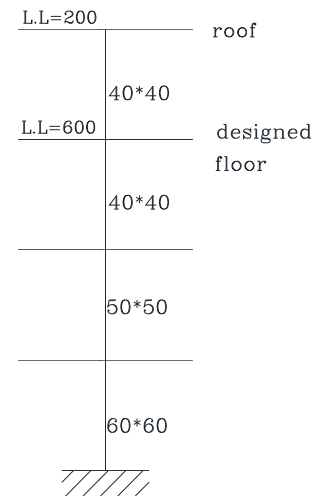
Column:

$$b = 30 \text{ cm (min)}$$

$$= \frac{H}{15} = \frac{4.5}{15} = 30 \text{ cm}$$

$$= \frac{L}{20} = \frac{7.5}{20} = 37.5 \text{ cm}$$

Use  $40 \times 40 \text{ cm}$

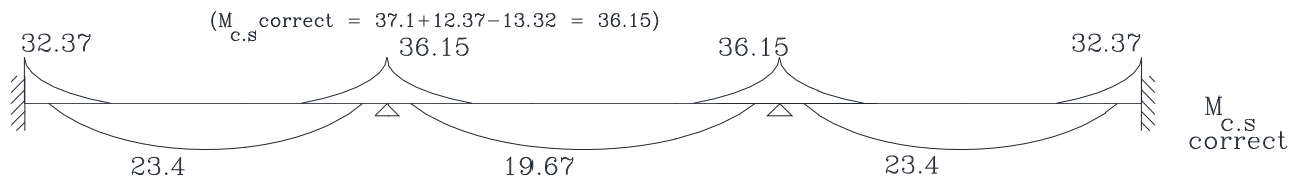


## Loads

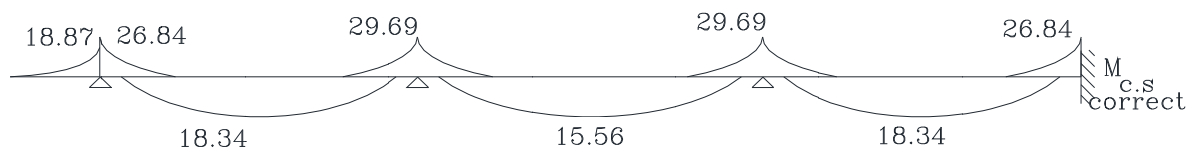
$$w_u = 19.4 \text{ kN/m}^2$$

## Calculation of B.M.D

Long direction



Short direction:





## Design of column

### Interior column (C1)

$$w_{u,roof} = 13 \text{ kN/m}^2$$

Sec1:

Case1:

$$\begin{aligned} N &= w_{u,roof} \times L_1 \times L_2 + o.w \\ &= 13 \times 7.5 \times 6.5 + 0.4 \times 0.4 \times 4.5 \times 25 \times 1.4 \\ &= 659 \text{ kN} \end{aligned}$$

$$\begin{aligned} k = 1.5, \lambda &= \frac{4.5 \times 1.5}{0.4} = 16.9 \rightarrow \delta = \frac{\lambda^2 \times b}{2000} \\ &= \frac{16.9^2 \times 0.4}{2000} = 0.057 \text{ m} \end{aligned}$$

$$M_{add} = N \cdot \delta = 659 \times 0.057 = 37.6 \text{ kN.m}$$

$$e = \frac{M}{N} = 0.057 \text{ m}, \quad \frac{e}{t} = \frac{0.057}{0.4} = 0.1425 \rightarrow \text{use interaction diagram}$$

$$\mu = 0.5\%, \quad \mu_{min} = 0.025 + 0.052\lambda_{max} = 1.13\%$$

$$A_s = \frac{1.13}{100} \times 400 \times 400 = 1808 \text{ mm}^2 \rightarrow 8\phi 18$$

Case2:

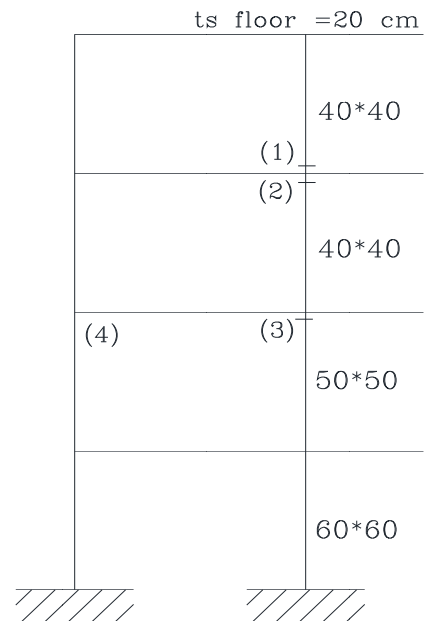
$$\begin{aligned} N &= (D.L. + 0.5 \times L.L.) \times L_1 \times L_2 + o.w \\ &= (9.8 + 0.5 \times 3.2) \times 7.5 \times 6.5 + 0.4 \times 0.4 \times 4.5 \times 25 \times 1.4 = 581 \text{ kN} \end{aligned}$$

$$M = 50\% M_{-ve} \text{ of col strip} \times \text{stiffness ratio} = 0.5 \times 361.5 \times 0.5 = 90.4 \text{ kN.m}$$

$$M_{add} = N \cdot \delta = 581 \times 0.057 = 33.2 \text{ kN.m} \rightarrow M_{total} = 90.4 + 33.2 = 123.6 \text{ kN.m}$$

$$e = \frac{M}{N} = 0.213 \text{ m}, \quad \frac{e}{t} = \frac{0.213}{0.4} = 0.53 \rightarrow \text{use interaction diagram} - \text{uniform steel}$$

$$\frac{N_u}{f_{cu} \times b \times t} = 0.14, \quad \frac{M_u}{f_{cu} \times b \times t^2} = 0.077 \rightarrow \rho = 4, A_s = \rho \cdot f_{cu} \cdot 10^{-4} \times b \times t = 1600 \text{ mm}^2$$





Case3:

$$N = 581 \text{ kN}$$

$$M = 50\%M_{-ve} \text{ of col strip} \times \text{stiffness ratio} \\ = 0.5 \times 296.9 \times 0.5 = 74.23 \text{ kN.m}$$

From Cases 1, 2 & 3  $\rightarrow A_s = 8\emptyset 18$

